

# The “Independence Principle” in the Processes of Water Transport

Julio A. Hernández\* and Jorge Fischbarg†

\*Sección Biofísica, Facultad de Ciencias, Universidad de la República, 11200 Montevideo, Uruguay, and †Departments of Physiology and Cellular Biophysics, and Ophthalmology, College of Physicians and Surgeons, Columbia University, New York, New York 10032 USA

**ABSTRACT** The processes of membrane transport exhibiting permeability coefficients depending on the species activities do not obey the “independence principle” and are assumed to take place by a mechanism of discrete nature, analyzable by a kinetic formalism. In this article, we study the dependence of the osmotic permeability coefficient on the water activities, from the steady-state analysis of a kinetic model of single-file water transport that simultaneously incorporates the vacancy-mediated and “knock-on” mechanisms into the state diagram. In particular, we study the relation between the near-equilibrium osmotic permeability ( $P_e$ ) and the equilibrium water activity of the compartments ( $w$ ). The analysis and numerical calculations performed for a simple case of the model show that, for values of the parameters consistent with experimental data,  $P_e$  exhibits only a small variation with  $w$  within the physiological range in the majority of the situations considered here. It is not possible to predict, from the study of these simple models, whether more complicated kinetic diagrams of water transport may be characterized by permeability coefficients with a more evident dependence on the water activities. Nevertheless, the results obtained here suggest that, for the case of physiological water pores, the analysis of the kinetic dependence of the permeability coefficients on the water activities may not yield evidence pointing to a discrete nature for the transport process.

## INTRODUCTION

The molecular nature of the mechanisms of water and solute transport through biological membrane pores remains unknown. Many studies of the theory of transport analyze a single-file model, one of the simplest mechanisms of pore transport. This mechanism has been proposed for diverse membrane systems; the gramicidin A pore constitutes the classical example (Finkelstein, 1987; Hille, 1992). To describe single-file water transport, two types of molecular models have been developed: continuum and discrete. Continuum models have included dynamical analysis under near-equilibrium conditions (Levitt, 1974; Finkelstein and Rosenberg, 1979) and linear thermodynamic analysis (Levitt, 1984). Both types of studies implicitly assume a “knock-on” mechanism (Hodgkin and Keynes, 1955). Discrete processes at the molecular level are usually represented by means of kinetic models. Kinetic models have been diversely employed to interpret properties in one-ion and multi-ion pores (see Hille, 1992), and in the special case of saturation conditions in ionic (Kohler and Heckmann, 1979, 1980; Schumaker and MacKinnon, 1990) and water (Hernández and Fischbarg, 1992) single-file pores.

The expressions for the permeability coefficients of the transported species reflect the basic difference between continuum and discrete mechanisms. Although in continuum models the permeability coefficients usually depend on a single system parameter (e.g., for the case of water, a viscosity coefficient) and are independent of the activities of the same and of different species, in discrete models they gen-

erally depend on more than one system parameter (e.g., several first-order rate constants) and are functions of the species activities. Thus, the “independence principle” applies to the former but not to the latter case (Schultz, 1980).

In a previous article (Hernández and Fischbarg, 1992), we derived explicit expressions for the osmotic and diffusive permeability coefficients of water from the analysis of a kinetic model of single-file water transport. Two different cases were considered: one-vacancy and “knock-on” (no-vacancy, or saturation) mechanisms. As expected (see above), the expressions derived there for the permeability coefficients in the case of vacancy-mediated transport are functions of the water activities. Instead, for the “knock-on” mechanism, the coefficients were shown to comply with the “independence principle.”

In this article, we explore the dependence of the osmotic permeability coefficient of a kinetic model for water transport that includes simultaneously in its diagram both vacancy and “knock-on” mechanisms. We term this a “complete” kinetic model of water transport. We seek to determine whether for physiological values of the water activities and osmotic permeability coefficients, such complete kinetic model may conform to the “independence principle.” With this purpose, we analyze here a complete kinetic model of single-file water transport.

In the first part, we discuss the complexity inherent to the study of this type of “complete” kinetic models of single-file transport, and we analyze the limiting behaviors of these models. In the second part, we study the dependence of the near-equilibrium osmotic permeability coefficient  $P_e$  on the equilibrium water activity. For illustration, we show numerical examples of this dependence for a particular simple case.

## “COMPLETE” KINETIC MODELS OF SINGLE-FILE TRANSPORT

In Fig. 1 we show the simplified state diagram for a “complete” kinetic description of water transport through a

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Address reprint requests to Dr. Jorge Fischbarg, Departments of Physiology Cellular Biophysics and Ophthalmology, College of Physicians and Surgeons, Columbia University, New York, NY 10032. Tel.: 212-305-9092; Fax: 212-305-2461; E-mail: fischbarg@cuccfa.ccc.columbia.edu.

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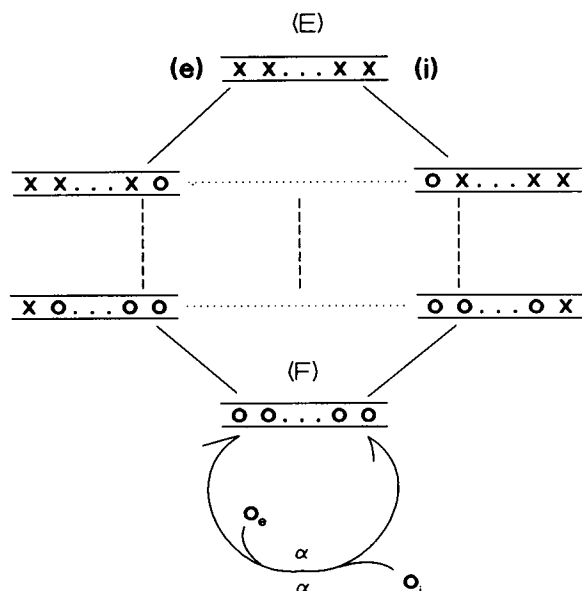


FIGURE 1 A simplified kinetic diagram showing vacancy-mediated and "knock-on" mechanisms of transport of a single species through a one-conformational single-file pore, between compartments "e" and "i". (X) Vacancies; (O) positions occupied by molecules. "E" and "F" are the completely empty and occupied states, respectively. In the vacancy portion, the diagram only shows the limiting cycles corresponding to single-occupancy and single-vacancy transport. In each cycle, the punctuated line represents a linear sequence of transitions. The network of states and transitions connecting these cycles is suggested by the broken lines. For an example showing this portion in detail, see Fig. 2. See also text for further details.

one-conformational single-file pore with a maximum of  $n$  occupancy sites in the file. The diagram includes the vacancy-mediated region (from one to  $n$  vacancies) and the single loop representing the "knock-on" mechanism (Hernández and Fischbarg, 1992). For simplicity, the vacancy region only shows the cycles for the single-occupancy and the single-vacancy limiting behaviors. States "E" and "F" in Fig. 1 are the states having  $n$  vacancies and no vacancies, respectively. Hence, the "knock-on" loop implies a transition between state "F" and itself. The detailed balance restriction requires that the same rate constant (" $\alpha$ ") will govern the frequency of transition in either direction. Here and elsewhere, we analyze the model of Fig. 1 by means of the graphic algorithm originated by King and Altman (1956) and modified by Hill (1977).

We assume that water traverses a membrane unit area through  $N$  identical pores that behave as in the model of Fig. 1. Hence, the osmotic permeability coefficient  $P_w$  of the membrane is given by

$$P_w = \left( \frac{N}{\sum} \right) \left( S_i + \sum_F \alpha \right). \quad (1)$$

In Eq. 1,  $S_i$  represents a sum, taken over all the cycles of the vacancy portion that determine net water transport, of terms of the form  $\sum_i \pi_i$ . For a particular cycle  $i$ ,  $\sum_i$  is the sum of all the appendages to the cycle and  $\pi_i$  is, from the detailed balance restriction, the product of all the rate constants in any of the two directions of the cycle. The term  $\sum_F$  is the sum

of all the directional diagrams that feed into state "F." The denominator  $\sum$  is the sum of all the directional diagrams of all the states of the model. Because the "knock-on" transition constitutes a cycle, it does not contribute with terms to  $\sum$ . Hence,  $\sum$  is given by the same expression, both for the "complete" model including the "knock-on" transition (Fig. 1) and for the model corresponding to the vacancy portion only. For the same reason,  $\sum_F$  also equals the corresponding term emerging from the vacancy portion of the diagram only.

The limiting behaviors of  $P_w$  (Eq. 1) as a function of the water activities in the two compartments ( $w_i$  and  $w_e$ ) can be analyzed in general terms:

a) When  $(w_i, w_e) \rightarrow 0$ ,  $P_w$  approaches the single-occupancy cycle limit, i.e.,  $P_w \rightarrow N \pi / \Omega_{so}$ . Here  $\pi$  is the product of all the rate constants of the single-occupancy cycle in any of the two directions and  $\Omega_{so}$  is the sum of all the directional diagrams of all the states corresponding to the single-occupancy cycle only.

b) When  $(w_i, w_e) \rightarrow \infty$ ,  $\sum \rightarrow \sum_F$ , because  $\sum_F$  contains the terms of the higher order in the water activities. Correspondingly,  $S_i$  contains terms of lower order in the water activities and, therefore, in this limit,  $NS_i/\sum \rightarrow 0$ , because it is characteristic of a saturation process. Hence, if  $\alpha$  is not negligible,  $P_w \rightarrow N\alpha$ , the osmotic permeability coefficient of a process mediated exclusively by a "knock-on" mechanism (Hernández and Fischbarg, 1992).

In Fig. 2 we show the state diagram of vacancy-mediated transport of a single species through a single-file pore, for the case that  $n = 3$ . A similar model has been employed, for instance, to interpret some basic general properties of ionic channels (Hille, 1975). The steady-state kinetic analysis of this type of model can be performed employing standard methods, like matrix inversion or diagrammatic techniques (Hill, 1977). The application of the method developed by Hill (1977) shows the complexity inherent in the analysis of the

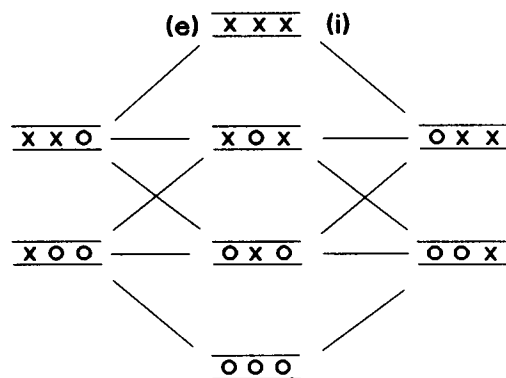


FIGURE 2 Kinetic model of vacancy-mediated transport of a single species through a one-conformational single-file pore containing three binding positions for the transported molecules. (X) Vacancies; (O) positions occupied by molecules. Compartment "e" is assumed to be on the left side, compartment "i" is on the right side of the pore. The solid lines represent actual transitions between the diagram states. The binding of molecules at the corresponding steps is not explicitly shown. See text for further details.

model of Fig. 2. Indeed, this model has 8 pore states, gives rise to 22 cycle diagrams, and contains more than 1000 directional diagrams, all of which make the derivation of explicit analytical expressions rather impractical. The analysis of the general case of  $n$  positions would certainly be even much more involved.

Kinetic analyses of single-file transport have been performed for the limiting case of one-vacancy pores (Kohler and Heckmann, 1979, 1980; Schumaker and MacKinnon, 1990; Hernández and Fischbarg, 1992). Explicit analytical studies of the kinetic model of vacancy-mediated single-file transport have already been done for the particular case that  $n = 2$ , for a pore having from 0 to 2 vacancies, and in the presence of one or two ionic ligands (Heckmann, 1965a, b, 1968; Urban and Hladky, 1979; Urban et al., 1980). These authors employed the algebraic procedure of successive elimination of unknowns to solve the velocity equations in steady state. For the purpose of our work, some basic ideas can be extracted from the analysis of the simple case  $n = 2$ . In the rest of this section, we show the basic properties of the analytical expressions for the "complete" model when  $n = 2$  (Fig. 3, and Appendix). We use the diagrammatic technique developed by Hill (1977).

From Eq. A2, the permeability coefficient  $P_w$  for the model of Fig. 3 is

$$P_w = \left( \frac{N}{\sum} \right) [\pi(\sum_1 + \sum_2) + \sum_F \alpha], \quad (2)$$

where  $\pi$ ,  $\sum$ ,  $\sum_1$ , and  $\sum_2$  are defined in the Appendix, and where  $N$ ,  $\sum_F$ , and  $\alpha$  have the same meanings as in Eq. 1. As can be seen,  $P_w$  depends on the activities  $w_e$  and  $w_i$  and, therefore, does not obey the "independence principle" (Schultz, 1980). Notice that if  $\alpha$  is negligible,  $P_w$  corresponds to the permeability coefficient of the vacancy model (see Eq. A2).

Under the condition

$$b_e w_e + b_i w_i \gg r_e + r_i \quad \text{and} \quad r_e r_i, \quad (3)$$

$$r_e^2 \quad \text{and} \quad r_i^2 \quad \text{negligible,}$$

the term  $\sum$  becomes

$$\sum = \Omega_2 \sum_2, \quad (4)$$

where  $\Omega_2$  is defined in the Appendix. Also from condition (3), we obtain

$$\sum_1 + \sum_2 = \sum_2. \quad (5)$$

From (4) and (5), Eq. 1 becomes

$$P_w = N \left[ \left( \frac{\pi}{\Omega_2} \right) + \left( \frac{\sum_F \alpha}{\sum} \right) \right]. \quad (6)$$

If  $\alpha$  is negligible, Eq. 6 becomes Eq. A11. Therefore, under condition (3) and for a negligible contribution of the "knock-on" mechanism, the kinetic properties of the model in Fig. 3 A are given by the single cycle 2 (Fig. 3 B). Cycle 2 represents the particular case  $n = 2$  of the general one-

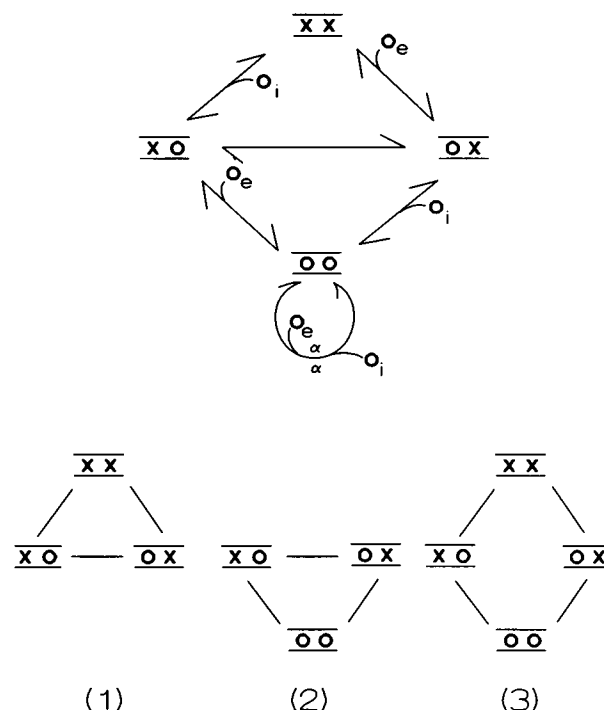


FIGURE 3 Kinetic model of transport of a single species through a single-file pore containing two binding positions for the transported molecules, including both the vacancy and "knock-on" mechanisms. Xs, open circles, and compartments are as in Fig. 2. Positions inside the pore are numbered "1" and "2" in the  $e$  to  $i$  direction. (A) The complete state diagram. The solid lines represent actual transitions between states. See text for the definition of the rate constants corresponding to the particular transitions. (B) The three cycles arising from the vacancy portion of the kinetic model of A.

vacancy single-file pore model, studied elsewhere as a model of multi-occupancy ionic or water pores (Kohler and Heckmann, 1979, 1980; Schumaker and MacKinnon, 1990; Hernández and Fischbarg, 1992).

Under the condition

$$r_e + r_i \gg b_e w_e + b_i w_i \quad \text{and} \quad b_e w_e b_i w_i, \quad (7)$$

$$(b_e w_e)^2 \quad \text{and} \quad (b_i w_i)^2 \quad \text{negligible,}$$

the term  $\sum$  becomes

$$\sum = \Omega_1 \sum_1, \quad (8)$$

where  $\Omega_1$  is defined in the Appendix. Also from condition (7) we obtain, in this case,

$$\sum_1 + \sum_2 = \sum_1. \quad (9)$$

Under condition (7),  $\sum_F$  becomes negligible. Therefore, from (8) and (9), Eq. 2 now becomes

$$P_w = N\pi/\Omega_1, \quad (10)$$

which is identical with Eq. A10. Therefore, under condition (7) the kinetic properties of the model in Fig. 3 A are given by the single cycle 1 (Fig. 3 B). Cycle 1 represents the particular case  $n = 2$  of the general single-occupancy pore model, diversely employed to describe ionic transport (for a review and references, see Hille, 1992).

For the case of a negligible contribution of the “knock-on” mechanism, conditions (3) and (7) therefore determine the limiting behaviors corresponding to single-vacancy and single-occupancy transport, respectively. It is possible that these basic conditions can be generalized to an arbitrary  $n$ . This is suggested by the general features and symmetry of the diagrams (see Figs. 2 and 3) and by some results on the asymptotic behavior of the multi-vacancy model under near-saturation conditions (Kohler and Heckmann, 1979, 1980). If  $\alpha$  is not negligible, the one-vacancy mechanism does not represent a good approximation to describe transport under near-saturation conditions. In this latter case, and for the simple example analyzed here,  $\alpha$  must appear in the treatment, and  $P_w$  would be given by Eq. 6, which expresses the simultaneous contribution of the one-vacancy and “knock-on” mechanisms. In other words, for near-saturating water activities, water transport through one-conformational single-file pores would take place by a mixture of discrete (one vacancy-mediated) and continuum-like (e.g., “knock-on”) regimes.

### OSMOTIC PERMEABILITY COEFFICIENT: DEPENDENCE ON THE WATER ACTIVITIES

For the case of one-conformational narrow pores, a plausible description of the transport process is provided by Fig. 1, and the permeability coefficient correspondingly is given by Eq. 1. As mentioned above, if  $\alpha$  is negligible, the permeability coefficient given by an expression of the type of Eq. 1 corresponds to a system exhibiting saturation kinetics, typical of a process that does not obey the “independence principle” (Schultz, 1980). Under this condition, the permeability coefficient should decrease with the species activities, as emerges from inspection of Eqs. 1 and 2. For water, this would imply a “non-Newtonian” type of behavior, because the osmotic permeability coefficient would then depend on the water activities (or the equivalent osmotic or hydrostatic pressures). Conversely, for water transport to comply with the independence principle constitutes the microscopical counterpart of the “Newtonian” behavior of classical hydrodynamics.

We first analyze in this section the relation between  $P_w$  and the equilibrium activity of water ( $w$ ) for the simple case under consideration (Fig. 3) and then show some numerical examples. As also commented above, the complexity of the diagrams representing larger pores restricts our study to the case that  $n = 2$ . Therefore, the contents of this section should mainly be considered as an example of a possible approach to the problem, and as an illustration of the behavior that actual biological water pores may exhibit.

To further simplify the study, we assume that

$$b_e = b_i = b; \quad r_e = r_i = r; \quad \text{and} \quad k_{12} = k_{21} = k. \quad (11)$$

Condition (11) corresponds to a “symmetrical” pore. Near equilibrium, we assume that  $w_e = w_i = w$ . In this case, and under condition (11), Eq. 2 becomes

$$P_e = (P_w)_{eq} = P_v + P_{ko}, \quad (12)$$

where  $P_v$  corresponds to the contribution of the vacancy portion to the overall near-equilibrium permeability coefficient  $P_e$ , and is given by

$$P_v = \frac{Nkrb}{(r + bw)(r + bw + 2k)}, \quad (13)$$

and where  $P_{ko}$ , provided by the “knock-on” mechanism, is given by

$$P_{ko} = \frac{Nb^2w^2\alpha}{(r + bw)^2}. \quad (14)$$

We now ask ourselves whether the determination of  $P_e$  at two “extreme” experimental values of  $w$ ,  $w_1$  and  $w_2$ , could put in evidence a noticeable difference between the corresponding permeability coefficients. For this purpose, we define

$$\Delta w = w_2 - w_1, \quad (15)$$

where  $w_1$  and  $w_2$  are two particular values of  $w$ , such that  $w_2 > w_1$ . Hence, from Eqs. 12–14, the difference between the corresponding near-equilibrium permeability coefficients,  $\Delta P_e$ , is given by

$$\Delta P_e = P_e(w_2) - P_e(w_1) = \Delta P_v + \Delta P_{ko}, \quad (16)$$

where, for this particular example (condition (11)),

$$\Delta P_v = Nbrk \left\{ \frac{1}{(r + bw_2)(r + bw_2 + 2k)} - \frac{1}{(r + bw_1)(r + bw_1 + 2k)} \right\}, \quad (17)$$

and

$$\Delta P_{ko} = Nb^2\alpha \left[ \frac{w_2^2}{(r + bw_2)^2} - \frac{w_1^2}{(r + bw_1)^2} \right]. \quad (18)$$

From Eqs. 15–18, we can express  $\Delta P_v$ ,  $\Delta P_{ko}$  and, therefore,  $\Delta P_e$  as functions of  $\Delta w$ ,  $w_1$ , and the parameters. We now consider two situations.

i)  $\alpha$  negligible.

In this case,  $\Delta P_e$  approximately corresponds to  $\Delta P_v$  that, for the example under consideration, is given by Eq. 17. From this expression and from definition (15), we see that  $\Delta P_v$  is of the form

$$\Delta P_v = - \frac{\Delta w(L_0 + L_1 w_1)}{M_0 + M_1 w_1 + M_2 w_1^2 + M_3 w_1^3 + M_4 w_1^4}, \quad (19)$$

where the  $L$ s and the  $M$ s are positive functions of  $\Delta w$  and of the parameters.

When

$$w_1 \rightarrow 0, \quad \Delta P_v \rightarrow - \frac{\Delta w L_0}{M_0}, \quad (20)$$

where

$$L_0 = Nbk r[2b(r + k) + b^2 \Delta w], \quad (21)$$

and

$$M_0 = r(2k + r) \times \{r(2k + r) + \Delta w[2b(r + k) + b^2 \Delta w]\}. \quad (22)$$

When

$$w_1 \rightarrow \infty, \quad \Delta P_v \rightarrow 0. \quad (23)$$

Hence, the limits (20) and (23) are characteristic of a transport process exhibiting saturation kinetics. Because from Eq. 19 the absolute value of  $\Delta P_v$  ( $\Delta P_e$  in this case) decreases monotonously with  $w_1$ , the maximum absolute value of  $\Delta P_v$  is achieved in the limit given by (20). From a biological point of view, this limit constitutes an unrealistic condition. In the vast majority of the situations, physiological water activities correspond to the saturation region. Therefore, a transport model of the type of Fig. 3 is likely to exhibit small (if not negligible) variations of the permeability coefficient as a function of the water activities, even in the case that the

basic mechanism of transport is mostly of discrete nature.

ii)  $\alpha$  not negligible.

In this case, the contribution of the term  $\Delta P_{k_0}$  to the overall difference  $\Delta P_e$  (Eq. 16) should be considered. From Eqs. 15 and 18, we see that the expression for  $\Delta P_{k_0}$  is of the form

$$\Delta P_{k_0} = \frac{\Delta w(J_0 + J_1 w_1 + J_2 w_1^2)}{K_0 + K_1 w_1 + K_2 w_1^2 + K_3 w_1^3 + K_4 w_1^4}, \quad (24)$$

where the  $J$ s and the  $K$ s are positive functions of  $\Delta w$  and of the parameters. Hence, Eq. 24 always represents a positive function.

When

$$w_1 \rightarrow 0, \quad \Delta P_{k_0} \rightarrow \frac{\Delta w J_0}{K_0}, \quad (25)$$

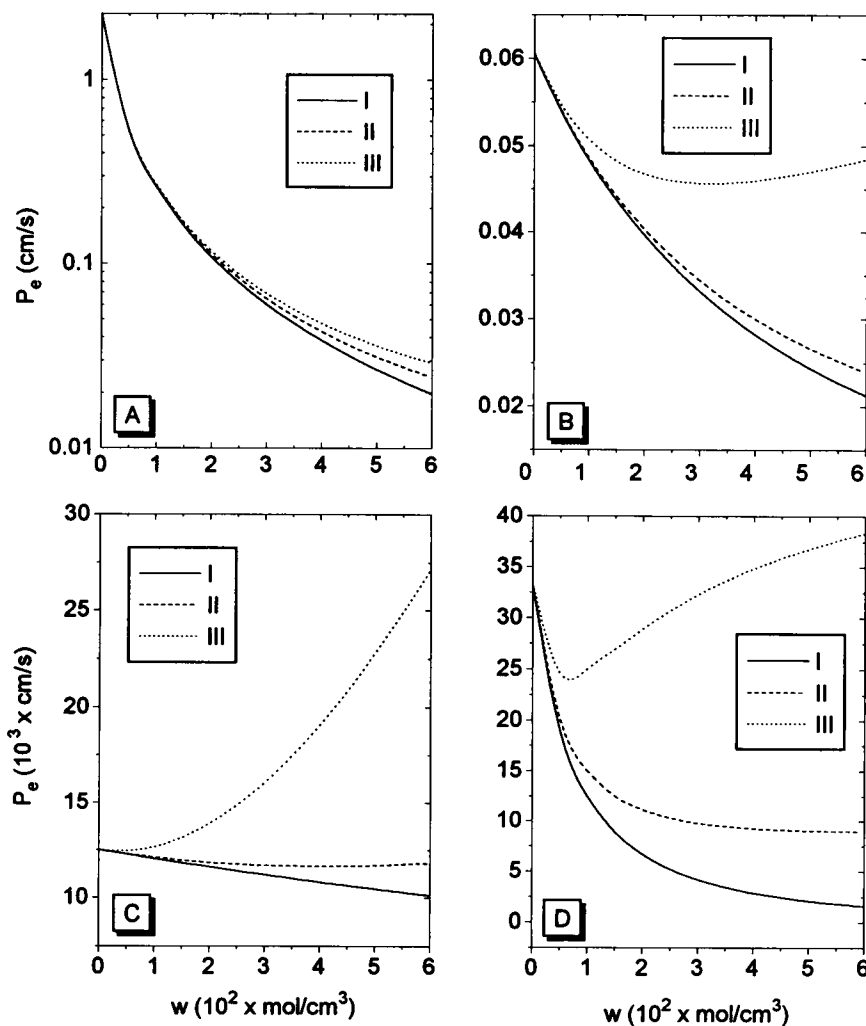
where

$$J_0 = N \alpha b^2 r^2 \Delta w, \quad (26)$$

and

$$K_0 = r^2[r^2 + \Delta w(2br + b^2 \Delta w)], \quad (27)$$

FIGURE 4 Plots of  $P_e$  as a function of  $w$ , using Eqs. 12–14, for different sets of values of the rate constants  $b$  ( $\text{cm}^3 \text{mol}^{-1} \text{s}^{-1}$ ),  $k$  ( $\text{s}^{-1}$ ),  $r$  ( $\text{s}^{-1}$ ), and  $\alpha$  ( $\text{cm}^3 \text{mol}^{-1} \text{s}^{-1}$ ). For all the curves,  $N = 10^{-12} \text{mol/cm}^2$ . (A)  $b = 5 \times 10^{12}$ ;  $k = 5 \times 10^{10}$ ;  $r = 10^{10}$ ;  $\alpha = 0$ (I),  $5 \times 10^9$ (II),  $10^{10}$ (III). (B)  $b = 10^{10}/0.055$ ;  $k = r = 10^{10}$ ;  $\alpha = 0$ (I),  $10^{10}$ (II),  $10^{11}$ (III). (C)  $b = 5 \times 10^{10}$ ;  $k = 10^{10}$ ;  $r = 2 \times 10^{10}$ ;  $\alpha = 0$ (I),  $10^{11}$ (II),  $10^{12}$ (III). (D)  $b = 10^{11}$ ;  $k = 10^9$ ;  $r = 10^9$ ;  $\alpha = 0$ (I),  $10^{10}$ (II),  $5 \times 10^{10}$ (III). See text for comments.



If  $\Delta w$  is sufficiently small, we can neglect terms in  $(\Delta w)^2$ . In this case, the contribution of the "knock-on" term ((25)) to the limit  $\Delta P_e(w_1 \rightarrow 0)$  is negligible, and this limit, therefore, is given by (20).

When

$$w_1 \rightarrow \infty, \quad \Delta P_{ko} \rightarrow 0. \quad (28)$$

Similarly to the absolute value of  $\Delta P_v$  (Eq. 19), Eq. 24 also decreases monotonously with  $w_1$ . Therefore, considering the limit given by (28), determinations of  $\Delta P_e$  performed at saturating  $w$ s are not likely to put in evidence significant differences in the permeability coefficients, independently of whether  $\alpha$  is negligible or not.

We employ the equations derived in the first part of this section to perform some numerical calculations. Typical values of pore-mediated osmotic permeability of the most water-permeable cells studied are of the order of  $10^{-2}$  cm/s (House, 1974; Finkelstein, 1987; Verkman, 1992). We also assume, for the calculations, that  $N$  is  $10^{-12}$  mol/cm<sup>2</sup>, corresponding approximately to  $6 \cdot 10^{11}$  pores per cm<sup>2</sup>, a plausible value for the number of water channels in the erythrocyte membrane (Denker et al., 1988; Preston et al., 1992). As-

suming these values, the water permeability per pore would be of the order of  $10^{-14}$  cm<sup>3</sup>/s, the approximate value predicted for the pore formed by gramicidin A (Finkelstein, 1987). To perform the calculations that follow, we consider that the amount of water transported through the lipid bilayer is negligible. The values assumed for the rate constants are arbitrary. The only criteria for their choice is the plausibility, from a physiological point of view, of the values obtained for the permeability coefficients as functions of the equilibrium water activity.

Figs. 4 and 5 show calculations corresponding to the "complete" model of Fig. 3, and for a wide range of values of the equilibrium water activity. In Figs. 6 and 7 we restricted the study to the behavior of the vacancy portion of the model only, and for a narrower, closer to physiological, range of the water activities.

Fig. 4 shows the dependence of  $P_e$  on the equilibrium water activity  $w$ , as given by Eqs. 12–14, for different sets of values of the rate constants. As can be seen, for the physiological values of  $P_e$  (about  $10^{-2}$  cm/s) and of  $w$  (about 0.055 mol/cm<sup>3</sup>), curves I, II, and III of Fig. 4 A, and curves I and II of Fig. 4 B–D, correspond to the saturation zone. In

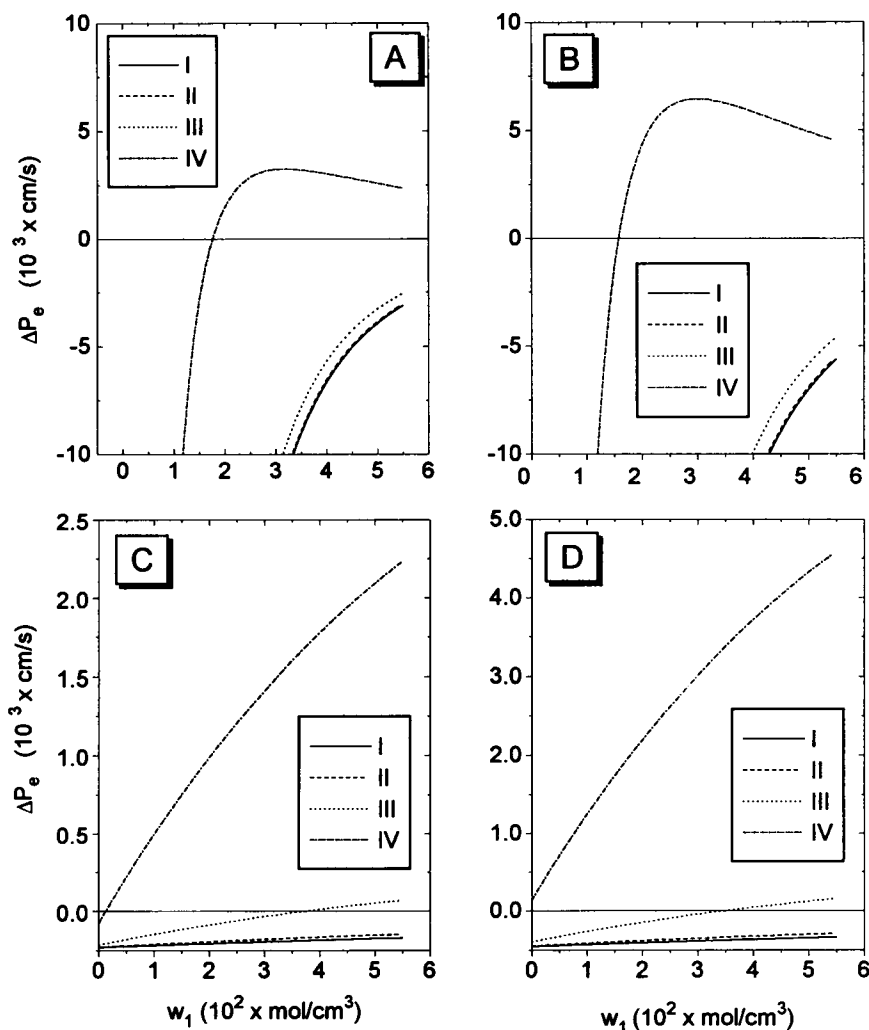


FIGURE 5 Plots of  $\Delta P_e$  as a function of  $w_1$ , using Eqs. 15–18, for different sets of values of the rate constants  $b$  (cm<sup>3</sup> mol<sup>-1</sup> s<sup>-1</sup>),  $k$  (s<sup>-1</sup>),  $r$  (s<sup>-1</sup>) and  $\alpha$  (cm<sup>3</sup> mol<sup>-1</sup> s<sup>-1</sup>), and of  $\Delta w$  (mol/cm<sup>3</sup>). (A and B)  $b = 5 \times 10^{12}$ ,  $k = 5 \times 10^{10}$ ,  $r = 10^{10}$ . (C and D)  $b = 5 \times 10^{10}$ ,  $k = 10^{10}$ ,  $r = 2 \times 10^{10}$ . (A and C)  $\Delta w = 0.005$ . (B and D)  $\Delta w = 0.01$ . For every plot  $\alpha = 0$ (I),  $10^{10}$ (II),  $10^{11}$ (III),  $10^{12}$ . (IV). See text for comments.

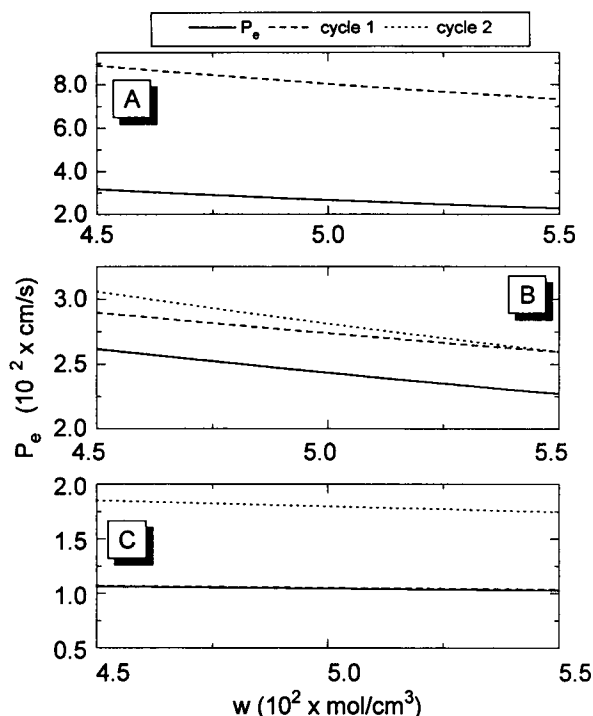


FIGURE 6 Plots of  $P_e$  as a function of  $w$  using Eq. 13, for different sets of values of the rate constants  $b$  ( $\text{cm}^3 \text{mol}^{-1} \text{s}^{-1}$ ),  $k$  ( $\text{s}^{-1}$ ), and  $r$  ( $\text{s}^{-1}$ ). Plots of Eqs. A10 and A11 under condition 11 and for equilibrium activities of  $w$ , corresponding to the permeability coefficients of the single cycles 1 and 2, respectively, are also included (curves 1 and 2, respectively). For all the curves  $N = 10^{-12} \text{ mol/cm}^2$ . (A)  $b = 5 \times 10^{12}$ ,  $k = 5 \times 10^{10}$ ,  $r = 10^{10}$ ; (6)  $b = 10^{10}/0.055$ ,  $k = r = 10^{10}$ ; (6)  $b = 5 \times 10^{10}$ ,  $k = 10^{10}$ ,  $r = 2 \times 10^{10}$ . See text for comments.

curve III of Fig. 4 B–D, the combination of values assigned to the rate constants yield “unphysiological,” namely too large, values of  $P_e$  in the physiological range of values of  $w$ .

Fig. 5 shows the dependence of  $\Delta P_e$  on the equilibrium water activity  $w_1$ , as given by Eqs. 16–18, for different sets of values of the rate constants and of  $\Delta w$ . Fig. 5, A and B on one side, and Fig. 5, C and D on the other, show plots performed using the same set of values of the rate constants, respectively. They only differ between them in the value employed for  $\Delta w$ . As can be seen, in an intermediate portion of the curves,  $\Delta P_e$  increases with  $\Delta w$ . To be noted, the absolute value of the difference  $\Delta P_e$  remains very small, even in the cases closest to physiological behavior. Curve IV of Fig. 5, C and D, showing a portion of the function notoriously increasing with  $w$ , corresponds to the “unphysiological” values of the rate constants that determine curve III of Fig. 4 C.

Fig. 6 shows the dependence of  $P_e$  on the near-equilibrium water activity  $w$  for different sets of values of the rate constants  $b$ ,  $k$ , and  $r$ , and for a negligible contribution of the “knock-on” mechanism (e.g.,  $\alpha = 0$ ). For each set of constants, we plot the permeability coefficients corresponding to the complete model ( $P_e$ ) as well as the permeability coefficients corresponding to the limiting cycles 1 and 2

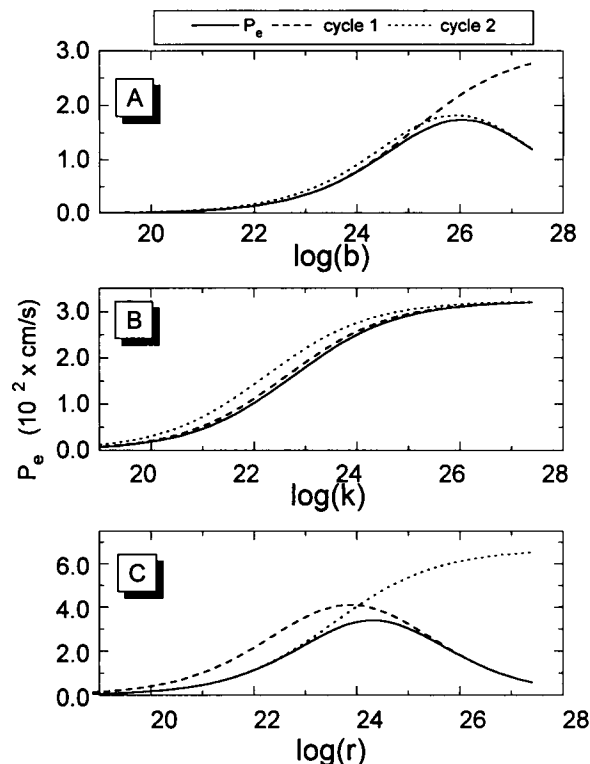


FIGURE 7 Plots of  $P_e$  as a function of the different rate constants  $b$  ( $\text{cm}^3 \text{mol}^{-1} \text{s}^{-1}$ ),  $k$  ( $\text{s}^{-1}$ ), and  $r$  ( $\text{s}^{-1}$ ), using Eq. 13. For all the curves,  $N = 10^{-12} \text{ mol/cm}^2$  and  $w = 0.055 \text{ mol/cm}^3$ . Plots of the permeability coefficients corresponding to cycles 1 and 2 are also included, as in Fig. 6. (A)  $k = 10^{10}$ ,  $r = 5 \times 10^9$ ; (B)  $b = 10^{11}$ ,  $r = 10^{10}$ ; (C)  $b = 5 \times 10^{11}$ ,  $k = 10^{10}$ . See text for comment.

(curves 1 and 2, respectively). This allows us to evaluate the accuracy of the single cycle approximations for the range of  $w$  considered. As can be seen, the model is able to display both non-Newtonian (Fig. 6, A and B) and quasi-Newtonian behavior (Fig. 6 C), depending on the particular values of the rate constants. However, considering the very large range of water activities studied, the observed variation of the permeability coefficients is very small, even in the most extreme cases depicted in Fig. 6, A and B. Superpositions occur between curve 2 and  $P_e$  in Fig. 6 A and between curve 1 and  $P_e$  in Fig. 6 C. In the former case, condition (3) applies, whereas condition (7) applies in the latter example.

Fig. 7 shows the dependence of  $P_e$  on each of the rate constants, for the cases of the vacancy region, and of the single cycles 1 and 2. As may be deduced from an analysis of Eq. 13,  $P_e$  increases and then decreases around a maximum, as a function of both  $r$  and  $b$  (Fig. 7, A and C, respectively). Therefore, in both of these cases,  $P_e$  is not single-valued, because similar values of  $P_e$  can be obtained for two different values of the corresponding rate constant. Also,  $P_e$  exhibits “saturation kinetics” as a function of  $k$  (Fig. 7 B). In this vacancy model, the resulting functions of  $P_e$  approximately correspond to a transition between the regimes determined by each one of the limiting cycles.

## DISCUSSION

Water channel proteins plus some transporters and channels have been demonstrated to constitute sites of water transport across cell membranes (nicotinic acetylcholine receptor (Dani, 1989); facilitative glucose transporters (Fischbarg et al., 1990; Zhang et al., 1991); CHIP28 water channel (Preston et al., 1992); Cystic Fibrosis transmembrane conductance regulator (Hasegawa et al., 1992)). Many of them may operate via narrow inner channels by means of a single-file mechanism. The model in Fig. 1 represents the most general description of the processes of transport taking place in one-conformational single-file pores, because it includes a variety of behaviors ranging from the single-occupancy to the fully saturated “knock-on” mechanisms. Thus, Eq. 1 expresses the permeability coefficient resulting from a co-existence of different regimes and allows one to derive particular limiting cases.

The results obtained in the first part of the present work show that, under near-saturating water activities, the permeability coefficients derived from the single-file water transport model reflect practically only the properties of the one-vacancy and full saturation (e.g., “knock-on”) modes. This analytical result agrees with the evidence that emerges from molecular dynamics simulations of gramicidin A pores (Chiou et al., 1989; Roux and Karplus, 1991; Chiu et al., 1993), which suggests that the nature of the water transport mechanism is intermediate between one-vacancy mediated and continuum processes. Furthermore, x-ray crystallography studies show that hydrophilic cavities in soluble proteins are either saturated with water or have at most one vacancy (see, for instance, the number of solvent sites reported for globular proteins in Hendrickson and Wüthrich, 1991–1994). From all of the above, analytical models of water transport through narrow pores at near-saturation apparently need only consider one-vacancy and full saturation mechanisms.

Both the analytical and numerical studies performed here show only a very small variation of the osmotic permeability coefficient as a function of the water activity in the physiological range. This suggests that water transport through narrow pores exhibits a “Newtonian-like” behavior of the permeability coefficient, characteristic of bulk flow of water in continuous media. It is difficult to predict whether more complicated models representing longer and wider pores would give rise to permeability coefficients that would not obey the “independence principle” in a noticeable fashion.

Another conclusion of this study, illustrated by the examples shown in Fig. 7, is that a particular value of a permeability coefficient can be associated with different numerical values of some of the rate constants. As also shown in the simulations of Fig. 7, such different values could correspond to different regimes of the transport process, which could not be identified by the numerical value of the permeability coefficient by itself.

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## APPENDIX

### Steady-state analysis of the single-file pore model for the case $n = 2$

We perform here the steady-state analysis of the vacancy portion of the model of Fig. 3, using the diagram method developed by Hill (1977). The following symbols represent:

$b_e, b_i$ :	rate constants of binding from compartments $e$ and $i$
$r_e, r_i$ :	rate constants of release to compartments $e$ and $i$
$k_{12}, k_{21}$ :	rate constants of forward and backward transitions between positions 1 and 2
$N$ :	total number of pores per unit area
$w_e, w_i$ :	activities of $w$ in compartments $e$ and $i$

The flux of  $w$ , taken positive in the  $e$  to  $i$  direction, is given by

$$J_w = J_1 - J_2, \quad (A1)$$

where  $J_1$  and  $J_2$  are the cycle fluxes corresponding to cycles 1 and 2, respectively (Fig. 3 B), taken positive in the clockwise direction. From (A1), we express  $J_w$  by

$$J_w = \left( \frac{N\pi}{\sum} \right) \left( \sum_1 + \sum_2 \right) (w_e - w_i), \quad (A2)$$

where, from the detailed balance restriction,

$$\pi = b_e k_{12} r_i = b_i k_{21} r_e. \quad (A3)$$

The terms  $\sum_1$  and  $\sum_2$  are the sums of the appendages to cycles 1 and 2, respectively, and are given by

$$\sum_1 = r_e + r_i; \quad \sum_2 = b_e w_e + b_i w_i. \quad (A4)$$

The denominator  $\sum$  is the sum of all the directional diagrams of all the states of the model and is given by

$$\begin{aligned} \sum = & (b_e w_e + b_i w_i + r_e + r_i) \times (r_e r_i + b_e w_e r_i + b_i w_i r_e + b_e w_e b_i w_i) \\ & + (k_{12} b_e w_e + k_{21} b_i w_i) (b_e w_e + b_i w_i) + (k_{12} r_i + k_{21} r_e) (r_e + r_i) \\ & + (b_e w_e + b_i w_i) (r_e + r_i) (k_{12} + k_{21}). \end{aligned} \quad (A5)$$

The net fluxes determined by each of the cycles 1 and 2 functioning separately are given by

$$(J_w)_1 = (N\pi/\Omega_1) (w_e - w_i), \quad (A6)$$

and

$$(J_w)_2 = (N\pi/\Omega_2) (w_e - w_i), \quad (A7)$$

where  $\Omega_1$  and  $\Omega_2$  are the sums of all the directional diagrams of all the states belonging to cycle 1 and cycle 2, respectively, and are given by

$$\begin{aligned} \Omega_1 = & r_e r_i + r_e (k_{21} + b_i w_i) + r_i (k_{12} + b_e w_e) \\ & + (k_{12} + k_{21}) (b_e w_e + b_i w_i), \end{aligned} \quad (A8)$$

and

$$\begin{aligned} \Omega_2 = & b_e w_e b_i w_i + b_e w_e (k_{12} + r_i) + b_i w_i (k_{21} + r_e) \\ & + (k_{12} + k_{21}) (r_e + r_i). \end{aligned} \quad (A9)$$

From Eqs. A6 and A7, the permeability coefficients characterizing the transport of  $w$  by cycles 1 and 2 functioning separately are, respectively,

$$(P_w)_1 = (N\pi/\Omega_1), \quad (A10)$$

and

$$(P_w)_2 = (N\pi/\Omega_2). \quad (A11)$$



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